

FOUNDATION NOTES

DESIGN

1.1 FOUNDATION DESIGN IS IN ACCORDANCE WITH THE 2003 INTERNATIONAL BUILDING CODE AND COH AMMENDMENTS.
DESIGN WIND LOAD 115 MPH (3 SECOND GUST).

1.2 FOUNDATION DESIGN HAS BEEN PREPARED WITHOUT THE BENEFIT OF A GEOTECHNICAL REPORT AND IS BASED UPON GENERAL KNOWLEDGE OF THE AREA SOIL TYPES AND ALLOWABLE BEARING PRESSURES AND DEPTHS.

SITWORK

2.1 SITE PREPARATION BENEATH THE SLAB SHALL BE IN ACCORDANCE WITH THE FOLLOWING MINIMUM REQUIREMENTS.

2.1.1 EXCAVATE ALL AREAS BENEATH SLAB TO A MINIMUM DEPTH OF 4 INCHES OR AS REQUIRED TO REMOVE TOP SOIL AND ORGANIC MATERIAL.

2.1.2 PROOF ROLL EXPOSED SUBGRADE. EXCAVATE, BACKFILL, AND COMPACT ANY SOFT SPOTS ENCOUNTERED OR TREE HOLES WITH SELECT FILL MATERIAL.

2.1.3 SCARIFY EXPOSED SUBGRADE TO 6 INCH DEPTH, ADD MOISTURE IF NECESSARY AND RECOMPACT TO 95 PERCENT OF THE MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D698.

2.1.4 BACKFILL EXCAVATED AREA WITH SELECT MATERIAL AS REQUIRED TO BRING TO GRADE. SELECT MATERIAL SHALL CONSIST OF NON-ACTIVE CLAYEY SAND OR SANDY CLAYS WITH A LIQUID LIMIT OF LESS THAN 35 AND A PLASTICITY INDEX BETWEEN 4 AND 12. BANK SAND SHALL NOT BE USED AS SELECT FILL.

2.1.5 PLACE SELECT FILL IN LOOSE LIFTS, NOT TO EXCEED 6 INCHES IN THICKNESS, AND COMPACT TO AT LEAST 95 PERCENT OF STANDARD PROCTOR DENSITY ASTM D698.

2.1.6 MAINTAIN SUBGRADE AND FILL MOISTURE CONTENTS AND DENSITY UNTIL SLAB IS PLACED.

2.1.7 SELECT FILL MATERIAL PROPERTIES AND FIELD MOISTURE DENSITY TESTS SHOULD BE PERFORMED BY A GEOTECHNICAL ENGINEER.

2.2 PROVIDE 2 INCH MINIMUM SAND LEVELING LAYER. SAND SHALL BE WELL COMPACTED BANK SAND OR OTHER CLEAN GRANULAR MATERIAL.

2.3 FINAL GRADE SHALL BE SLOPED AWAY FROM THE FOUNDATION AT THE RATE OF 12 INCHES IN THE FIRST 10 FEET WITH A MINIMUM SLOPE OF 1/4 INCH PER FOOT THEREAFTER, SUCH THAT POSITIVE DRAINAGE AWAY FROM THE SLAB IS ASSURED.

2.4 PROVIDE CONTINUOUS 6 MIL MINIMUM THICKNESS, POLYETHYLENE VAPOR BARRIER BENEATH SLAB. ALL JOINTS SHALL BE TAPED.

CONCRETE

3.1 ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" ACI-318, LATEST EDITION.

3.2 CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI. WATER SHALL NOT BE ADDED TO CONCRETE AT THE JOB SITE. ADDITIONAL WORKABILITY SHALL BE OBTAINED BY AIR ENTRAINMENT OR OTHER ADMIXTURES AT THE BATCH PLANT. CALCIUM CHLORIDE IN ANY FORM SHALL NOT BE USED. FLY ASH SHALL NOT BE USED WITHOUT PRIOR APPROVAL OF THE ENGINEER.

3.3 CONCRETE SHALL NOT BE PLACED WHEN AMBIENT AIR TEMPERATURE IS 40 DEGREES F. AND FALLING, IN RAINY OR OTHER ADVERSE WEATHER CONDITIONS.

3.4 CONCRETE SHALL BE THOROUGHLY CONSOLIDATED DURING PLACEMENT.

3.5 FORMS SHALL NOT BE STRIPPED WITHIN 24 HOURS OF CONCRETE PLACEMENT.

3.6 ALL DIMENSIONS, DROPS, OFFSETS, BRICK LEDGES AND SLOPES SHALL BE VERIFIED WITH THE ARCHITECTURAL DRAWINGS PRIOR TO CONCRETE PLACEMENT.

3.7 PRIOR TO PLACING NEW CONCRETE AGAINST EXISTING FOUNDATION, COAT ALL SURFACES OF EXISTING EXISTING CONCRETE THAT WILL BE IN CONTACT WITH NEW CONCRETE WITH A BONDING AGENT.

REINFORCING

4.1 REINFORCING STEEL SHALL CONFORM TO THE FOLLOWING:

- #5 AND LARGER - ASTM A615, GRADE 60
- #3 AND #4 - ASTM A615, GRADE 40 OR 60

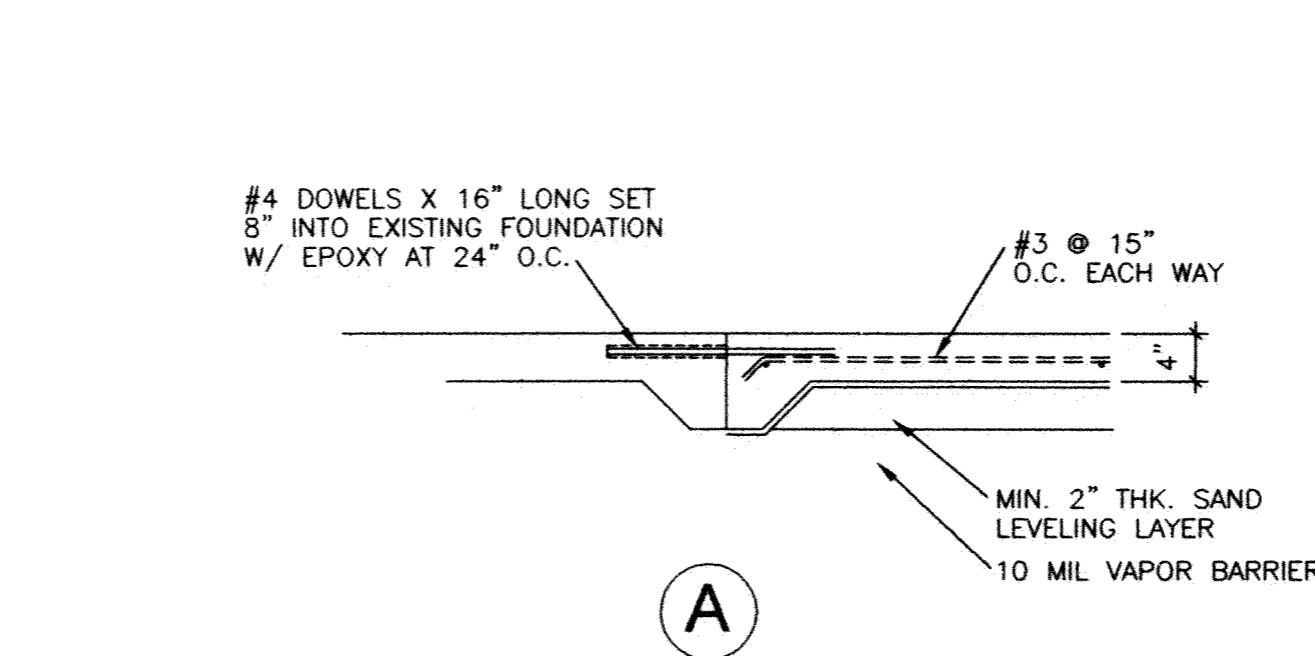
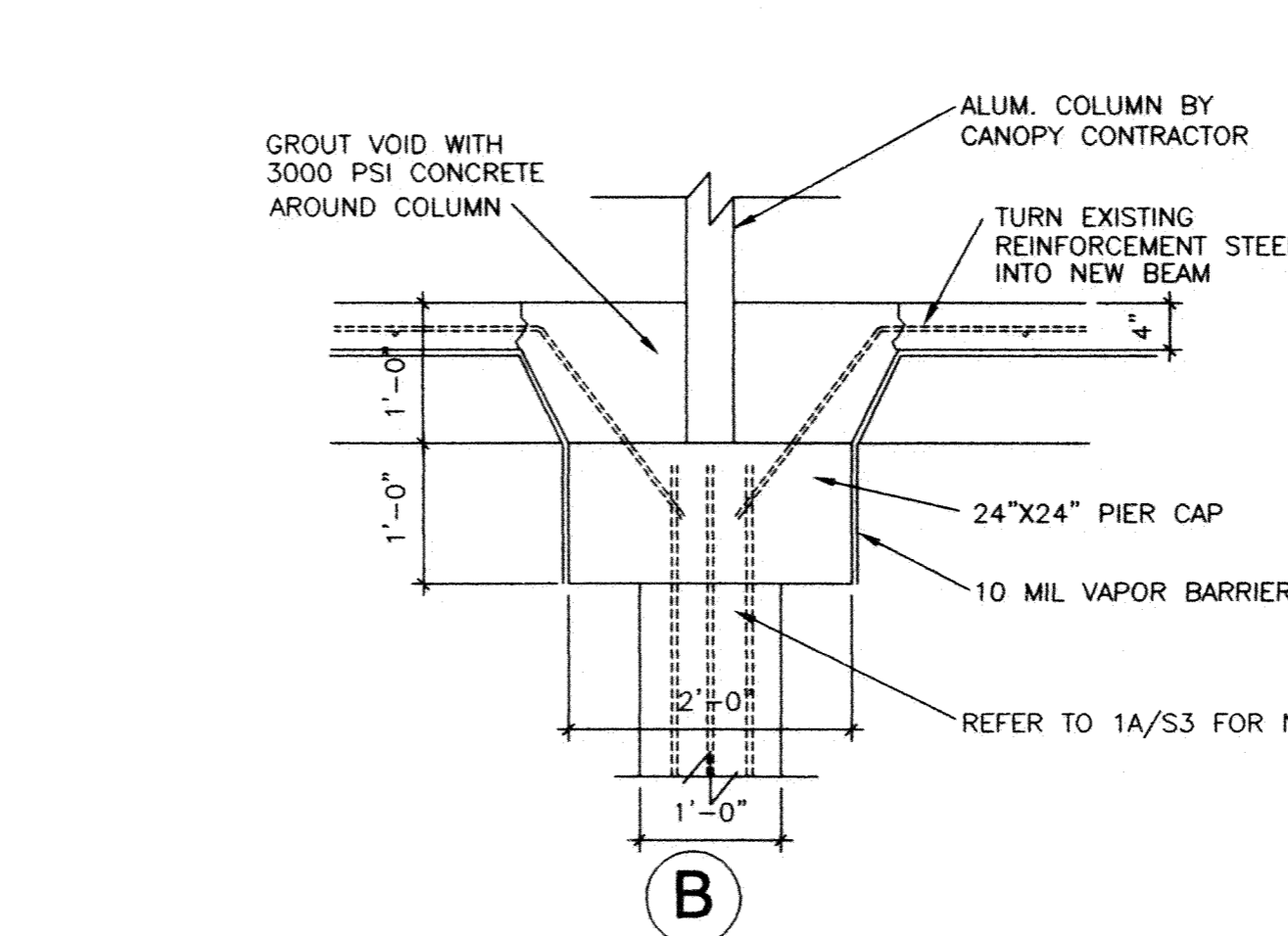
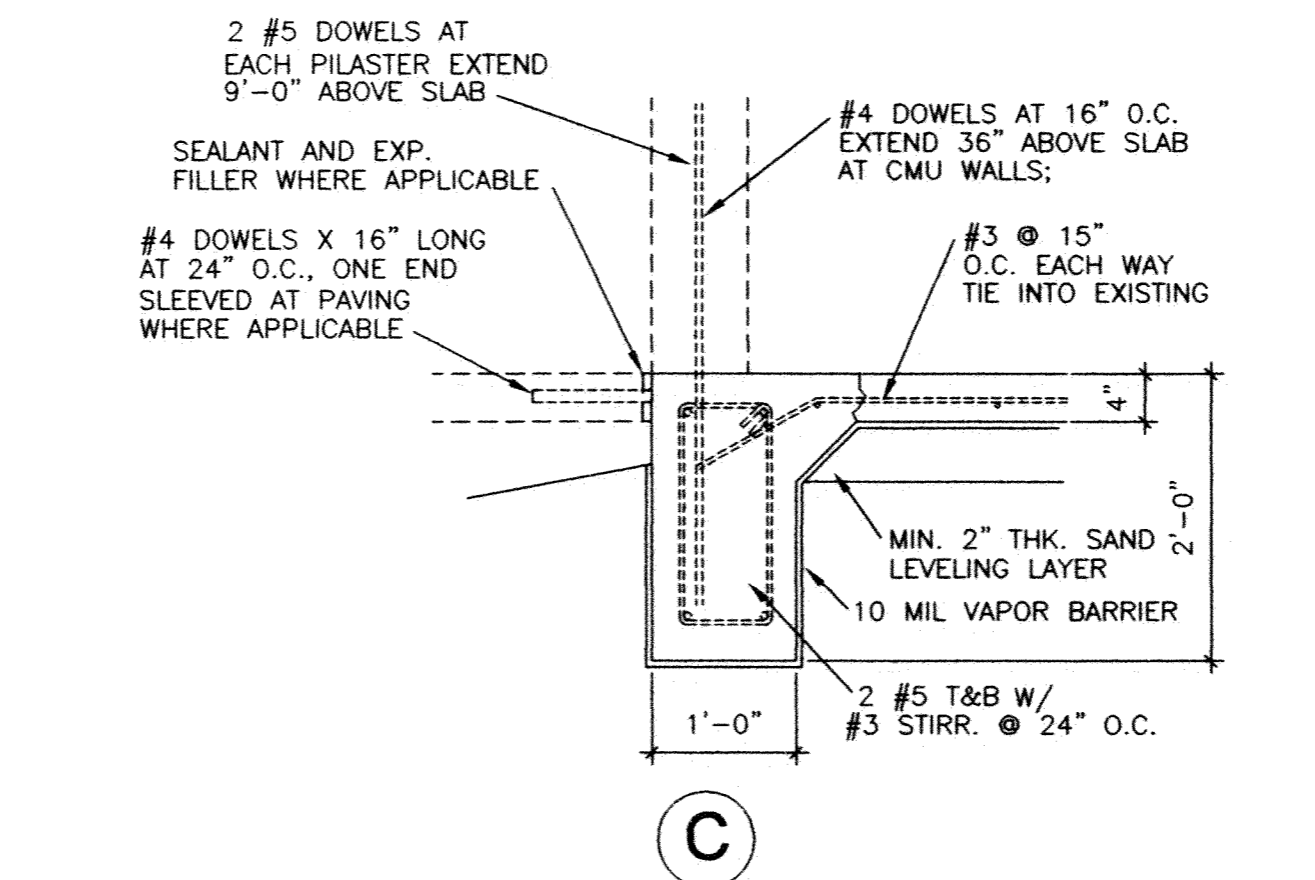
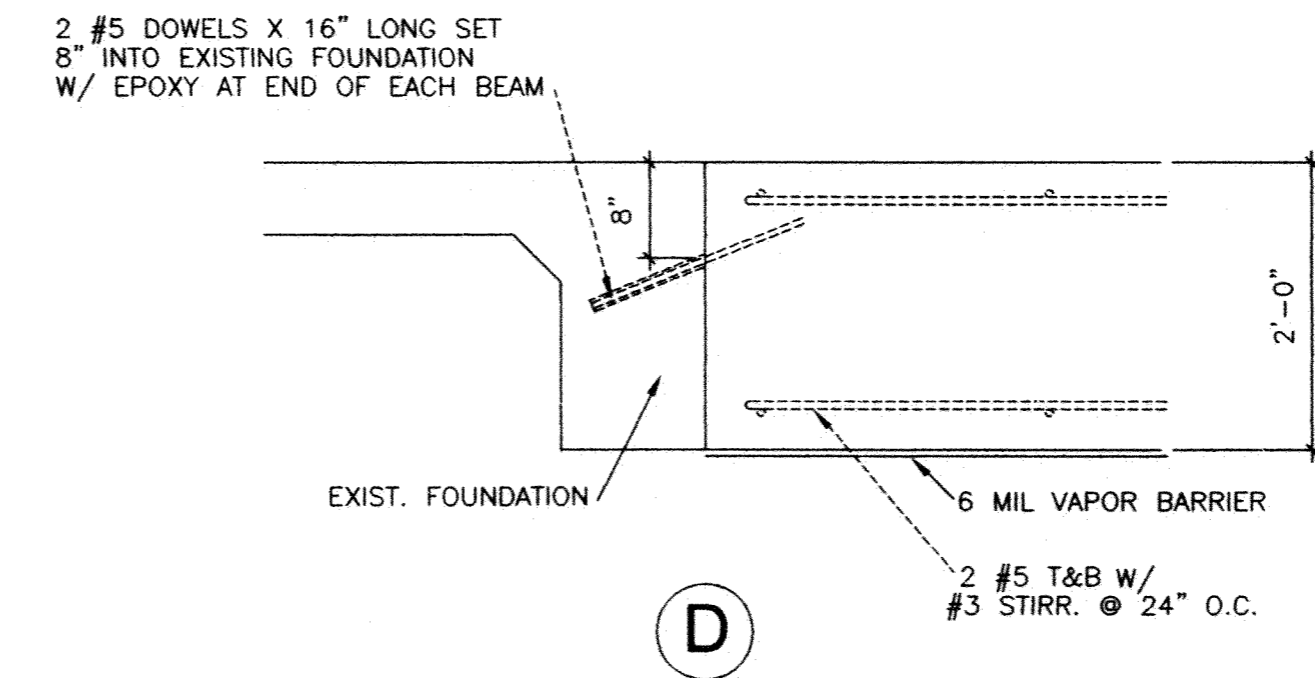
4.2 FIELD SPLICES SHALL BE LAPPED 30 BAR DIAMETERS MINIMUM. LAPS SHOULD BE STAGGERED. IN GRADE BEAMS, LAP TOP BARS AT MIDSPAN BETWEEN FOOTINGS AND BOTTOM BARS AT CENTER LINE OF FOOTINGS.

4.3 IN GRADE BEAMS, PROVIDE CORNER BARS OF THE SAME SIZE AND WITH 2"-0" LEGS AT EXTERIOR FACE OF BEAMS.

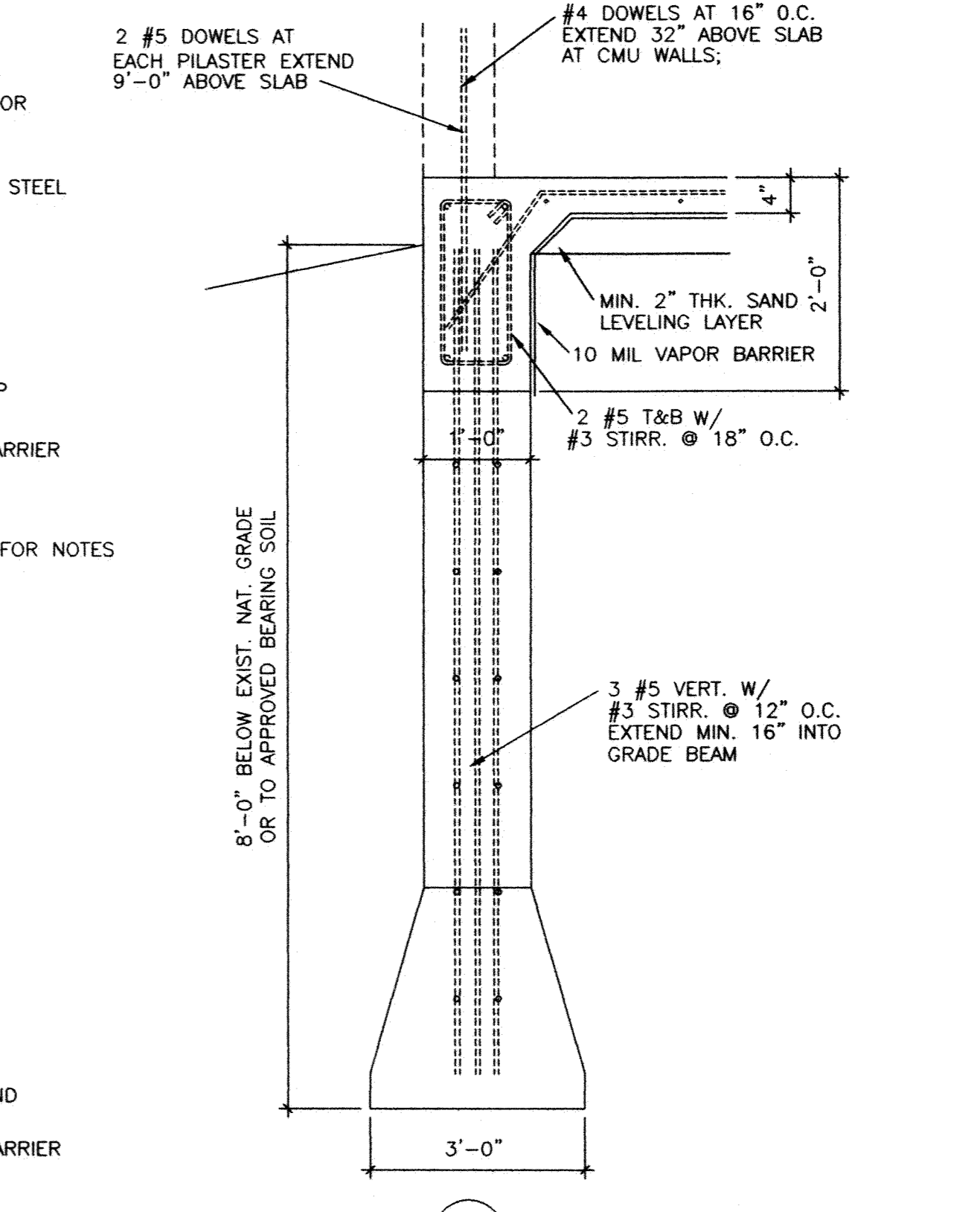
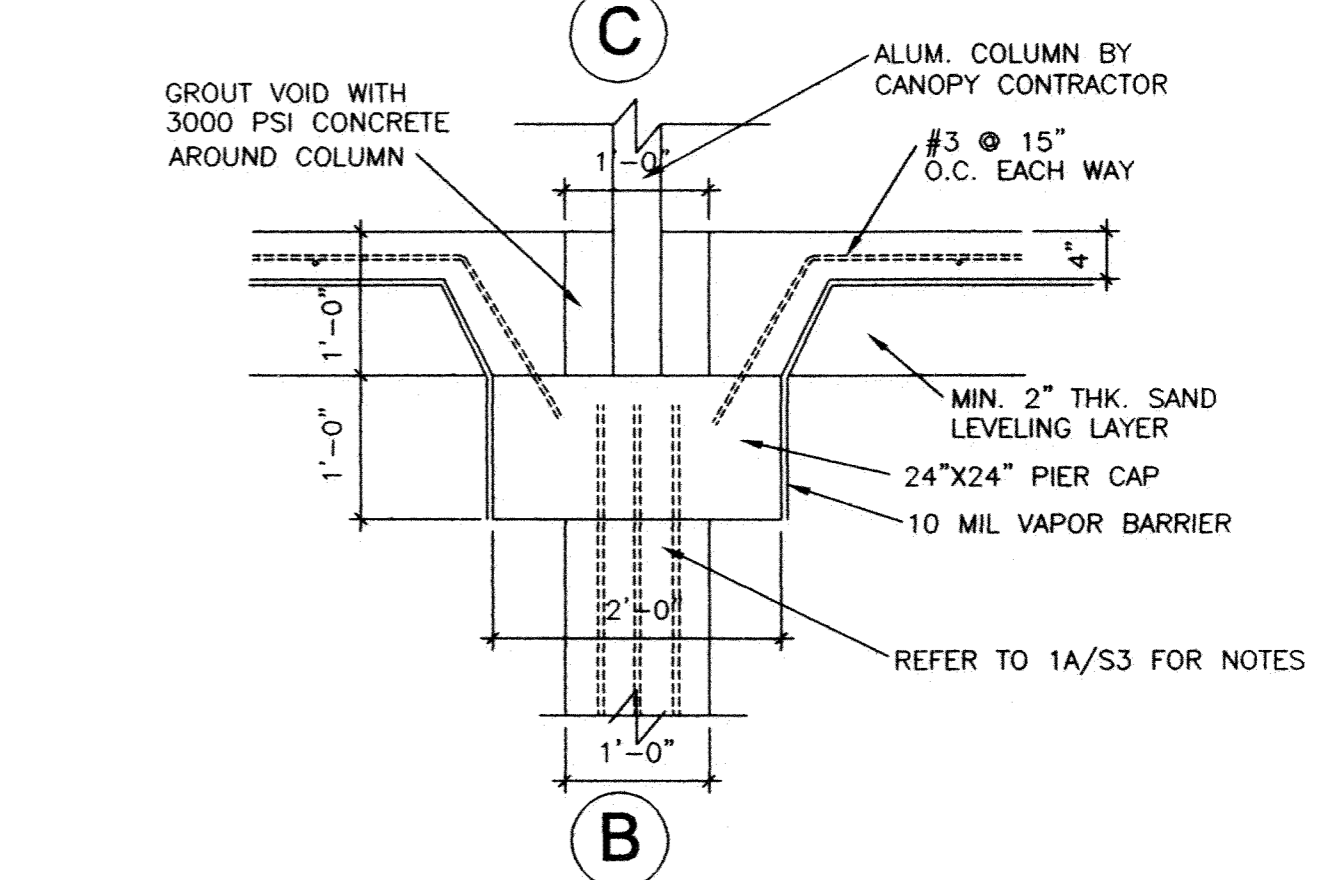
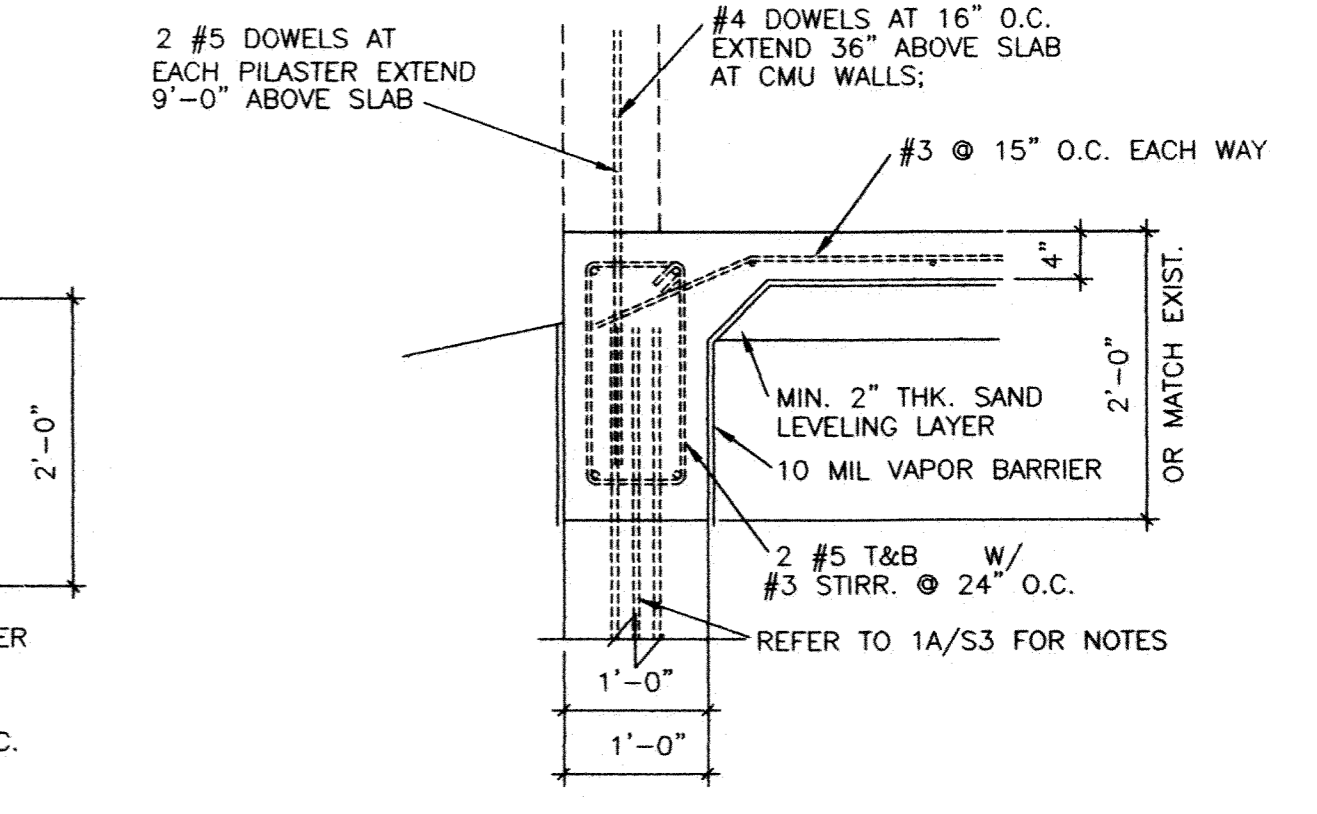
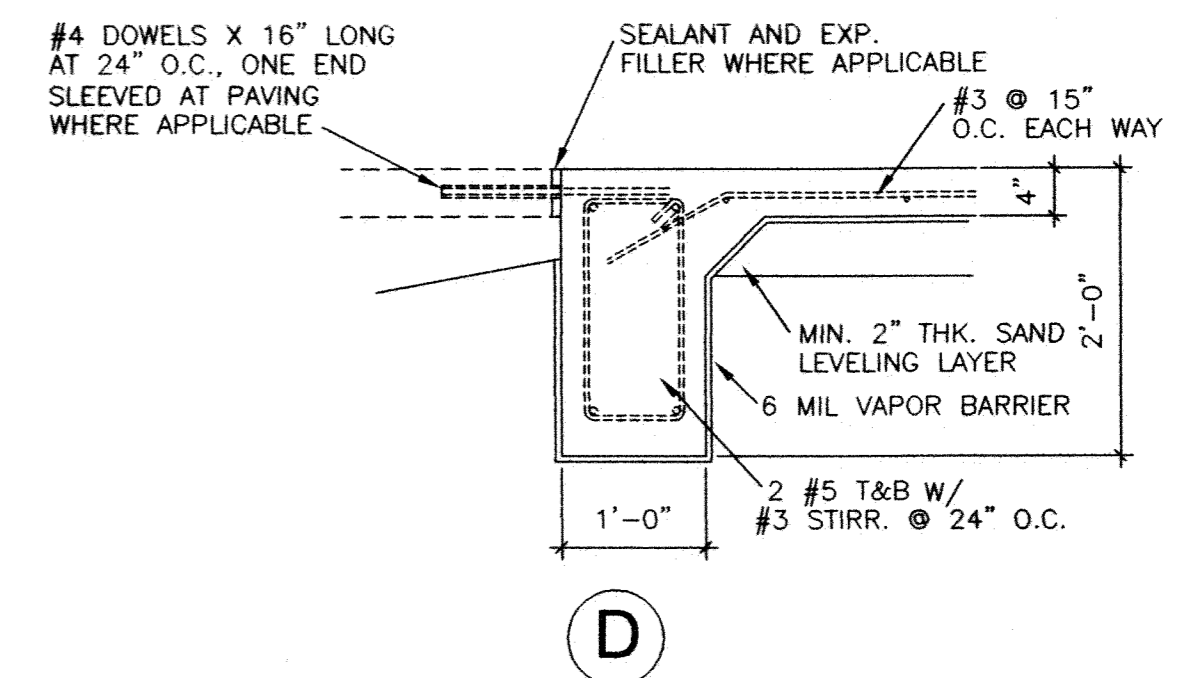
4.4 PROVIDE MINIMUM CONCRETE COVER AS FOLLOWS:

- CAST AGAINST EARTH - 3 INCHES
- EXPOSED TO EARTH OR WEATHER - 2 INCHES
- OTHER BARS - 1 1/2 INCHES

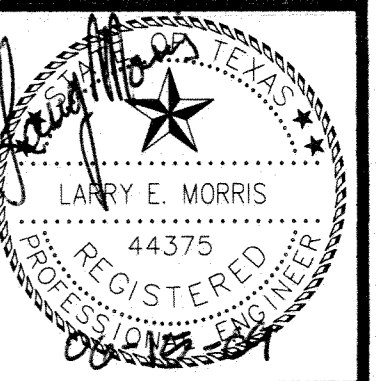
4.5 PROVIDE CHAIR SUPPORTS AT 4'-0" MAXIMUM ON CENTER FOR BARS.



2 FOUNDATION DETAILS
3/4"=1'-0"

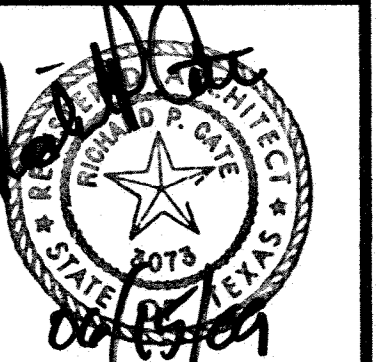


1 FOUNDATION DETAILS
3/4"=1'-0"



ARCHITECTS

HOUSTON, TEXAS
713-523-6070



**RENOVATIONS AT MHMRA'S
BRISTOW FACILITY
2627 CAROLINE HOUSTON, TEXAS**

PROJECT
09-026(A)
DATE
06/15/09
DRAWN BY

REVISIONS

SHEET
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OF